

FACULTY	AGRICULTURE, ENGINEERING, AND NATURAL SCIENCES		
SCHOOL	ENGINEERING AND THE BUILT ENVIRONMENT		
DEPARTMENT	CIVIL AND MINING ENGINEERING		
SUBJECT	DESIGN OF STEEL AND TIMBER STRUCTURES		
SUBJECT CODE	TCVS3762		
ASSESSMENT TYPE	SECOND SEMESTER EXAMINATION		
DATE	NOVEMBER 2023		
TIME ALLOWED	2 HOURS	MARKS	100

FIRST OPPORTUNITY EXAMINATION

Examiner: PROF. CHINWUBA ARUM

Internal Moderator: DR. PHILEMON ARITO

External Moderator: PROF. AKPOFURE TAIGBENU (University of the Witwatersrand)

This question paper consists of ...**21**.... pages including this front page.

Instructions

- (a) **Closed** book examination.
- (b) Read the questions carefully.
- (c) Timber design must be carried out according to the provisions of BS 5268 and steel design according to EUROCODE 3.
- (d) Fifteen (15) pages of code extracts and other relevant data are attached to this question paper for your convenient reference (please refer to pages 7 to 21).
- (e) Answer questions 1 and 2, plus EITHER question 3 OR question 4.
- (f) Marks for each question are indicated.
- (g) Start each question on a new page.

Question 1**[20 marks]**

A timber beam with a clear span of 3.65 m supports a uniformly distributed load of 3.5 kN/m including self-weight of the beam. The beam is made of timber strength class *C24* under service class 1. The bearing length is 150 mm and the ends of the beam are held in position and compression edge held in line. By using the BS 5268 procedure, for this beam:

- (a) Determine the minimum required section modulus Z_{xx} (assume $h = 225\text{ mm}$). **[6 marks]**
- (b) By considering only the bending deflection and ignoring the deflection due to shear, find the required second moment of area, I_{xx} , if the permissible deflection δ_p is given as:
 $\delta_p = 0.003 \times \text{span}$. **[5 marks]**
- (c) Using the section modulus, Z_{xx} , determined in (a) above and the second moment of area I_{xx} determined in (b), select a section for the timber beam which is adequate for bending and deflection. **[2 marks]**
- (d) Check the adequacy of the selected section for lateral buckling. **[3 marks]**
- (e) Check the adequacy of the section in shear. **[4 marks]**

Question 2**[20 marks]**

The ends of a $125 \times 75 \times 10$ single unequal angle tension member are connected to gusset plates through the larger leg by a single row of three 20 mm bolts in line in 22 mm holes at 70 mm centres. The gross cross-sectional area of the angle is $A = 19.1\text{ cm}^2$ and its thickness is $t = 10\text{ mm}$. If the angle is of grade *S355* steel and it is expected to resist a design tension force, $N_{t,Ed} = 440\text{ kN}$:

- (a) Determine the design plastic resistance of the gross cross-section **[5 marks]**
- (b) Find the design ultimate resistance of the net cross-section **[10 marks]**
- (c) What is the tension capacity of the angle? **[3 marks]**
- (d) Make a statement on the sufficiency of the capacity of the angle member to resist the given design tension force **[2 marks]**

Question 3**[60 marks]**

Fig.Q3 shows a typical floor plan of the steel framework of a multistory building. The **two-way spanning floor slab** provided is assumed **not able to provide lateral restraint to the compression flanges of the beams**. The *beams* are *simply-supported*. The load is not destabilizing. The following design data apply:

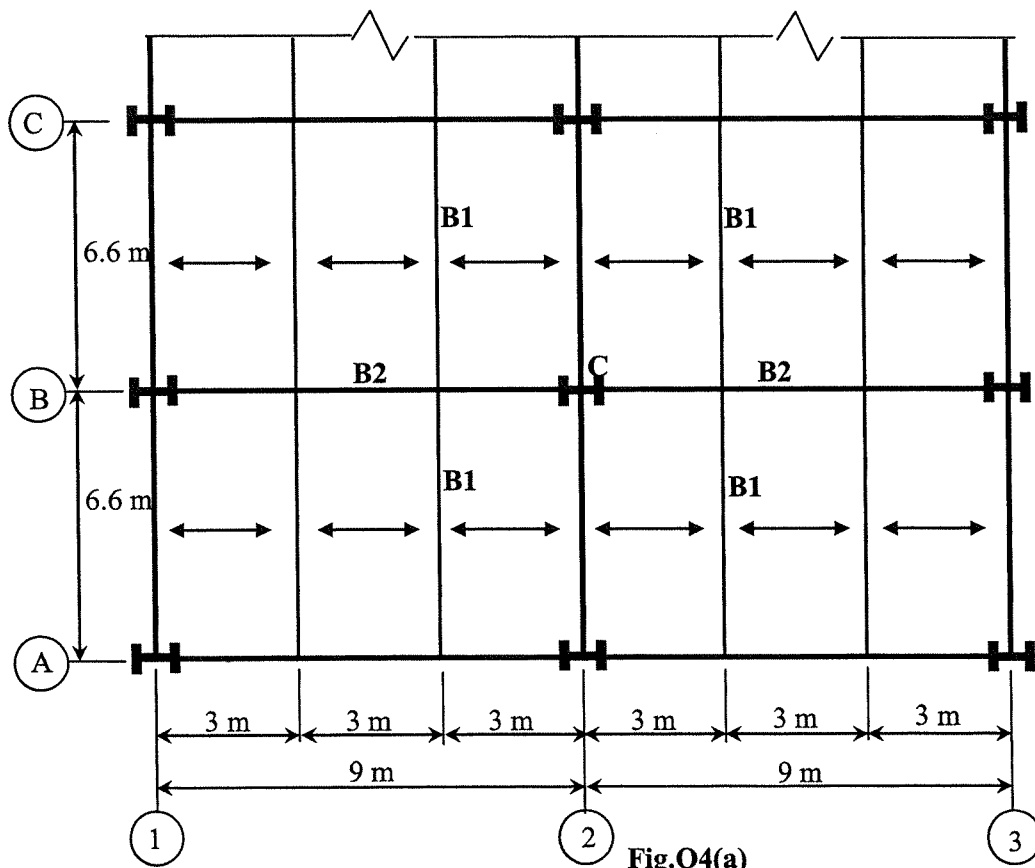
Design Data:

Total <i>characteristic</i> permanent action on the floor (including self-weight of beam)	4.5 kN/m^2
Total <i>characteristic</i> variable load on the floor	2.5 kN/m^2
Modulus of elasticity of steel	$2.1 \times 10^5 \text{ N/mm}^2$
Shear modulus of steel	$G = 8.1 \times 10^4 \text{ N/mm}^2$

Using the above **design data**, for the simply-supported **Beam B2-B3**:

- (a) Determine the ultimate load **[5 marks]**
- (b) Draw the shear force and bending moment diagrams and hence, indicate the design shear force and the design bending moment **[14 marks]**
- (c) Select a trial UB section in grade S355 steel and classify the cross-section **[10 marks]**
- (d) Check the bending resistance of the cross-section **[4 marks]**
- (e) Carry out lateral-torsional buckling check for the beam using either the EC 3 “**General Case**” or the “**Rolled or Equivalent Welded Section Case**” **[20 marks]**
- (f) Make a statement on the suitability of the trial section you selected for use as beam **B2-3** **[2 marks]**
- (g) If the selected section is not adequate, select one that is adequate, ensuring in all cases that the buckling resistance moment of the finally adopted beam is not greater than the design bending moment by more than 25%. **[5 marks]**

- (b) Determine the reactions at the supports for the simply-supported primary beam **B2** (span equals 9 m) [12 marks]
- (c) Determine the design axial load on the column at section **a-a** [7 marks]
- (d) Determine the design axial load on the column at section **b-b** [7 marks]
- (e) By using steel grade S355 and assuming that the buckling length $L_{cr} = 0.85L$, use the axial load obtained at section **b-b** to design a UC column for the first floor according to the following steps:
- Obtain a target area and hence select a trial UC section, listing all relevant section properties with their values. [4 marks]
 - Determine the relevant buckling curve and the imperfection factor α . [8 marks]
 - Calculate the reduction factor χ and hence determine the buckling resistance of the section. Make a statement on the adequacy or otherwise of the selected section. [10 marks]



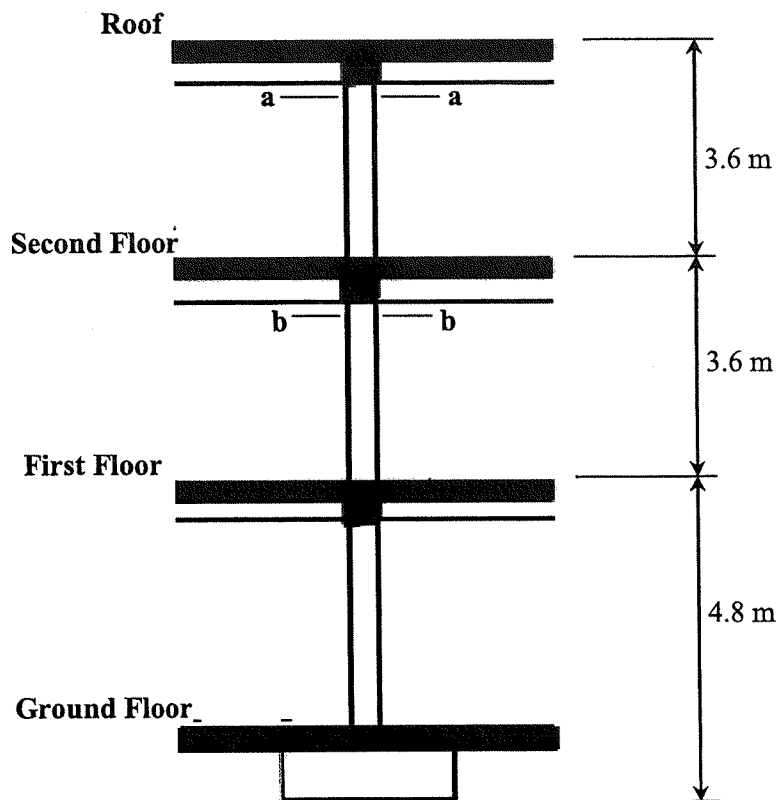


Fig.Q4(b)

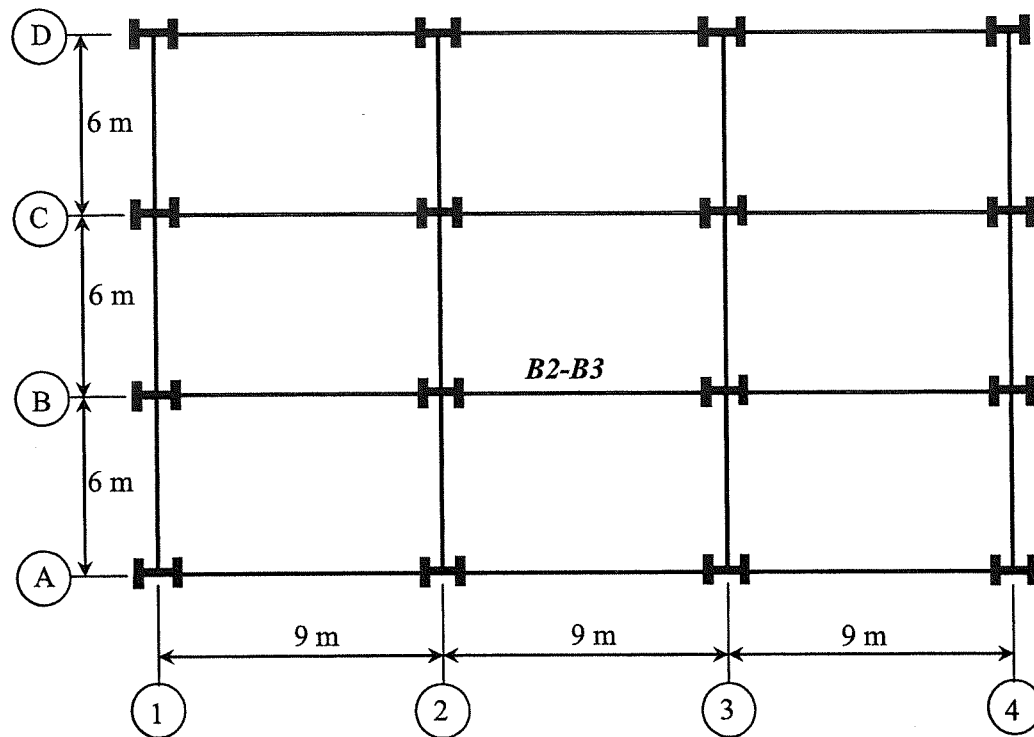


Fig.Q3

Question 4**[60 marks]**

Fig. Q4(a) represents the part-floor plan of an office building while **Fig.Q4(b)** is the elevation of the internal column stack C (on Grid Line B-2 on plan). The slabs for the floor and roof are prefabricated one-way spanning slabs and the roof and the floors have the same layout. The following design data apply:

Design Data:

Total characteristic permanent load on roof	$g_k = 4.0 \text{ kN/m}^2$
Total characteristic variable load on roof	$q_k = 1.5 \text{ kN/m}^2$
Total characteristic permanent load on floor	$g_k = 6.4 \text{ kN/m}^2$
Total characteristic variable load on floor	$q_k = 2.5 \text{ kN/m}^2$
Self-weight of the columns (including fire protection)	1.2 kN/m

Using the above information:

- (a) Determine the reactions at the supports for the simply-supported secondary beam B1 (span equals 6.6 m) **[12 marks]**

Table 5 Maximum depth to breadth ratios (Table 19, BS 5268)

<i>Degree of lateral support</i>	<i>Maximum depth to breadth ratio</i>
No lateral support	2
Ends held in position	3
Ends held in position and member held in line, as by purlins or tie-rods at centres not more than 30 times the breadth of the member	4
Ends held in position and compression edge held in line, as by direct connection of sheathing, deck or joists	5
Ends held in position and compression edge held in line, as by direct connection of sheathing, deck or joists, together with adequate bridging or blocking spaced at intervals not exceeding six times the depth	6
Ends held in position and both edges held firmly in line	7

Depth factor, K_7 :

$K_7 = 1.17$, for solid beams having a depth $h \leq 72\text{mm}$

$$K_7 = \left(\frac{300}{h} \right)^{0.11}, \text{ for solid beams with } 72\text{mm} < h < 300\text{mm}$$

$$K_7 = \frac{0.81(h^2 + 92300)}{(h^2 + 56800)}, \text{ for solid beams with } h > 300\text{mm}$$

$$\sigma_{m,adm,||} = \sigma_{m,g,||} K_3 K_7$$

$$Z_{xx,req} \geq \frac{M}{\sigma_{m,adm,||}}$$

$$\tau_a \leq \tau_{adm}, \text{ where:}$$

τ_a is the applied shear stress parallel to the grain; and

τ_{adm} is the permissible shear stress

$$\tau_a = \frac{3F_v}{2A}$$

$$\tau_{adm} = \tau_g K_3, \text{ where:}$$

τ_g is the grade shear stress parallel to the grain.

Table 4 Bending and shear deflections assuming $G = E/16$

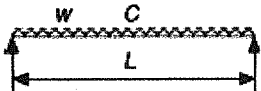
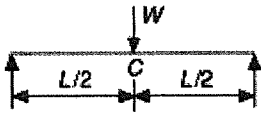
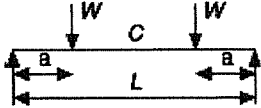
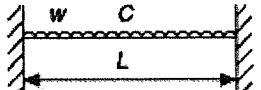
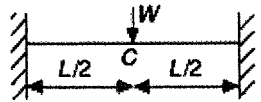
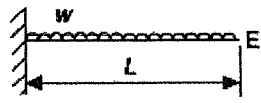
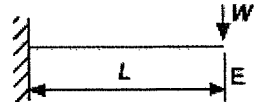
Load distribution and supports	Deflection at Centre C or end E	
	Bending	Shear
	$\frac{5}{384} \times \frac{wL^4}{EI}$	$\frac{12}{5} \times \frac{wL^2}{EA}$
	$\frac{WL^3}{48EI}$	$\frac{24}{5} \times \frac{WL}{EA}$
	$\frac{Wa}{EI} \left[\frac{L^2}{8} - \frac{a^2}{6} \right]$	$\frac{96}{5} \times \frac{Wa}{EA}$
	$\frac{wL^4}{384EI}$	$\frac{12}{5} \times \frac{wL^2}{EA}$
	$\frac{WL^3}{192EI}$	$\frac{24}{5} \times \frac{WL}{EA}$
	$\frac{wL^4}{8EI}$	$\frac{48}{5} \times \frac{wL^2}{EA}$
	$\frac{WL^3}{3EI}$	$\frac{96}{5} \times \frac{WL}{EA}$

Table 3 Geometrical properties of sawn softwoods

Customary target size*	Area	Section Modulus		Second of moment area		Radius of gyration	
		About x-x	About y-y	About x-x	About y-y	About x-x	About y-y
mm	10^3mm^2	10^3mm^3	10^3mm^3	10^6mm^4	10^6mm^4	mm	mm
22 × 100	2.20	36.6	8.1	1.83	0.089	28.9	6.35
38 × 100	3.80	63.3	24.1	3.17	0.457	28.9	11.0
38 × 150	5.70	143	36.1	10.7	0.686	43.3	11.0
38 × 175	6.54	194	42.1	17.0	0.800	50.5	11.0
38 × 200	7.60	253	48.1	25.3	0.915	57.7	11.0
38 × 225	8.55	321	54.2	36.1	1.03	65.0	11.0
47 × 75	3.53	44.1	27.6	1.65	0.649	21.7	13.6
47 × 100	4.70	78.3	36.8	3.92	0.865	28.9	13.6
47 × 125	5.88	122	46.0	7.65	1.08	36.1	13.6
47 × 150	7.05	176	55.2	13.2	1.30	43.3	13.6
47 × 175	8.23	240	64.4	21.0	1.51	50.5	13.6
47 × 200	9.40	313	73.6	31.3	1.73	57.7	13.6
47 × 225	10.6	397	82.8	44.6	1.95	65.0	13.6
47 × 250	11.8	490	92.0	61.2	2.16	72.2	13.6
47 × 300	14.1	705	110	106	2.60	86.6	13.6
63 × 150	9.45	236	99.2	17.7	3.13	43.3	18.2
63 × 175	11.0	322	116	28.1	3.65	50.5	18.2
63 × 200	12.6	420	132	42.0	4.17	57.7	18.2
63 × 225	14.2	532	149	59.8	4.69	65.0	18.2

Customary target size*	Area	Section Modulus		Second moment of area		Radius of gyration	
		About x-x	About y-y	About x-x	About y-y	About x-x	About y-y
mm	10^3mm^2	10^3mm^3	10^3mm^3	10^6mm^4	10^6mm^4	mm	mm
75 × 100	7.50	125	93.8	6.25	3.52	28.9	21.7
75 × 150	11.3	281	141	21.1	5.27	43.3	21.7
75 × 175	13.1	383	164	33.5	6.15	50.5	21.7
75 × 200	15.0	500	188	50.0	7.03	57.7	21.7
75 × 225	16.9	633	211	71.2	7.91	65.0	21.7
75 × 250	18.8	781	234	97.7	8.79	72.2	21.7
75 × 300	22.5	1130	281	169	10.5	86.6	21.7
100 × 100	10.0	167	167	8.33	8.33	28.9	28.9
100 × 150	15.0	375	250	28.1	12.5	43.3	28.9
100 × 200	20.0	667	333	66.7	16.7	57.7	28.9
100 × 225	22.5	844	375	94.9	18.8	65.0	28.9
100 × 250	25.0	1010	417	130	20.8	72.2	28.9
100 × 300	30.0	1500	500	225	25.0	86.6	28.9
150 × 150	20.0	563	563	42.2	42.2	43.3	43.3
150 × 300	30.0	2250	1130	338	84.4	86.6	43.3
300 × 300	90.0	4500	4500	675	675	86.6	86.6

Note. * Desired size of timber measured at 20% moisture content

**TABLES AND OTHER DATA FOR DESIGN OF STEEL AND TIMBER STRUCTURES FOR
“TCVS3762 DESIGN OF STEEL AND TIMBER STRUCTURES”
FIRST OPPORTUNITY EXAM NOVEMBER 2023**

I. TIMBER

Table 1 Grade stress and modulus of elasticity for strength class C24 (N/mm^2): for service classes 1 and 2 (based on BS 5268)

Bending parallel to grain $\sigma_{m,g, }$	Shear parallel to grain τ_g	Modulus of elasticity E_{min}
7.5	0.71	7200

Table 2 Modification factors K_3 for duration of loading (Table 17, BS 5268)

Duration of loading	Value of K_3
Long term (e.g. dead + permanent imposed ^a)	1.00
Medium term (e.g. dead + snow, dead + temporary imposed)	1.25
Short term (e.g. dead + imposed + wind ^b , dead + imposed + snow + wind ^b)	1.50
Very short term (e.g. dead + imposed + wind ^c)	1.75

Notes. ^a For uniformly distributed imposed floor loads $K_3 = 1.00$ except for type C3 occupancy (Table 1, BS 6399: Part 1: 1996) where for foot traffic on corridors, hallways, landings and stairways only, K_3 may be assumed to be 1.5.

^b For wind where the largest diagonal dimension of the loaded area a , as defined in BS 6399: Part 2, exceeds 50 m.

^c For wind, very short-term category applies to classes A and B (3 s or 5 s gust) as defined in CP3 : Chapter V : Part 2 or, where the largest diagonal dimension of the loaded area a , as defined in BS 6399: Part 2, does not exceed 50 m.

II. STEEL

A. CONNECTION DESIGN

The design plastic resistance of the gross cross-section is given by Eqn (1):

$$N_{pl,Rd} = \frac{A f_y}{\gamma_{M0}} \quad (\gamma_{M0} = 1.0) \quad (1)$$

For a single angle in tension connected by a single row of bolts in one leg, the design ultimate resistance of the net cross-section depends on the number of bolts and the pitch p_1 , and in general it is given by Eqn (2):

$$N_{u,Rd} = \frac{\beta A_{net} f_u}{\gamma_{M2}} \quad (\gamma_{M2} = 1.25) \quad (2)$$

where $\beta = \beta_2$ for two bolts, and $\beta = \beta_3$ for three bolts.

Table S1: Reduction factors β_2 and β_3 (Table 3.8 of EC3-1-8)

Pitch p_1		$\leq 2.5d_0$	$\geq 5.0d_0$
2 bolts β_2		0.4	0.7
3 bolts or more β_3		0.5	0.7

The symbols p_1 (pitch) and d_0 (hole diameter) in table S1 are defined in Fig. S1.

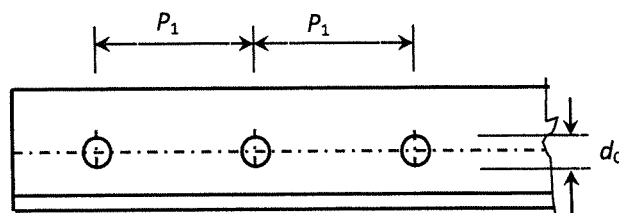


Fig. S1: Definition of symbols for tension connections in single angles

Table S2: Some nominal values of yield strength for hot-rolled steel (Table 3.1, EN 1993-1-1(2005))

Standard and steel grade	Nominal thickness of the element t [mm]			
	$t \leq 40$ mm		$40 \text{ mm} < t \leq 80$ mm	
	f_y [N/mm ²]	f_u [N/mm ²]	f_y [N/mm ²]	f_u [N/mm ²]
EN 10025-2				
S 235	235	360	215	360
S 275	275	430	255	410
S 355	355	490	335	470
S 450	440	550	410	550

B. BEAM DESIGN

For class 1 and class 2 cross-sections:

$$M_{c,y,Rd} = \frac{W_{pl,y} f_y}{\gamma_{M0}}$$

$$M_{cr} = C_1 \frac{\pi^2 EI_z}{L_{cr}^2} \sqrt{\left(\frac{I_w}{I_z} + \frac{L_{cr}^2 GI_T}{\pi^2 EI_z} \right)}$$

where C_1 is obtained from Table S3.

$$E = 2.10 \times 10^5 \text{ N/mm}^2; \quad G = 8.10 \times 10^4 \text{ N/mm}^2$$

$$\bar{\lambda} = \sqrt{\frac{W_y f_y}{M_{cr}}}$$

For a cross-section the buckling curve is selected from Table S4 and the imperfection factor α_{LT} from Table S5.

Table S3 C_1 values for transverse loading

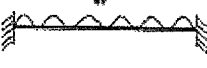
Loading and support conditions	Bending moment diagram	Value of C_1
		1.132
		1.285
		1.365
		1.565
		1.046

Table S4: LTB curves for general cases

Cross section	Limits	Buckling curve
Rolled I sections	$h/b \leq 2$	a
	$h/b > 2$	b
Welded I sections	$h/b \leq 2$	c
	$h/b > 2$	d
Other cross sections	—	d

Table S5: Imperfection factors for LTB curves

Buckling curve	a	b	c	d
Imperfection factor α_{LT}	0.21	0.34	0.49	0.76

$$\phi_{LT} = 0.5 \left[1 + \alpha_{LT} (\bar{\lambda}_{LT} - 0.2) + \bar{\lambda}_{LT}^2 \right]$$

$$\chi_{LT} = \frac{1}{\phi_{LT} + \sqrt{\phi_{LT}^2 - \bar{\lambda}_{LT}^2}} \text{ but } \chi_{LT} \leq 1.0$$

$$M_{b,y,Rd} = \chi_{LT} \frac{W_{pl,y} f_y}{\gamma_{M1}}$$

C. COLUMN DESIGN

Target area, $A \geq \frac{N \times \gamma_{M1}}{\chi \times f_y}$ where $\gamma_{M1} = 1.0$

For class 3 cross-sections:

$$\frac{c}{t} \leq 14\varepsilon \quad \text{for flanges;}$$

$$\frac{c}{t} \leq 42\varepsilon \quad \text{for webs, and}$$

$$\varepsilon = \left(\frac{235}{f_y} \right)^{1/2}$$

$$\lambda = \frac{L_{cr}}{i}$$

$$\bar{\lambda} = \frac{\lambda}{\lambda_1}, \quad \text{where } \lambda_1 = 93.9\varepsilon$$

Buckling curve is obtained from Table S6 and the imperfection factor α is obtained from Table S7.

$$\phi = 0.5 \left[1 + \alpha (\bar{\lambda} - 0.2) + \bar{\lambda}^2 \right]$$

$$\chi = \frac{1}{\phi + \sqrt{\phi^2 - \bar{\lambda}^2}}$$

$$N_{b,y,Rd} = \frac{\chi \times A f_y}{\gamma_{M1}}$$

Table S6 Selection of buckling curve for a cross-section

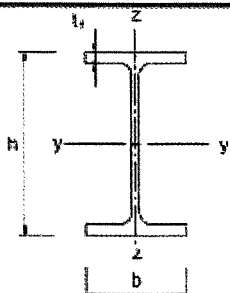
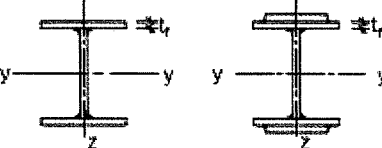
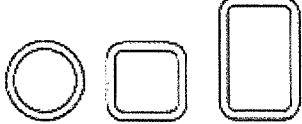
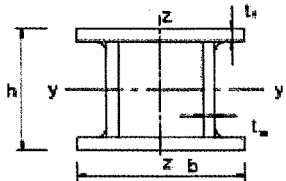
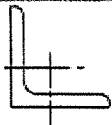
Cross section		Limits		Buckling about axis	Buckling curve	
					S 235 S 275 S 355 S 420	S 460
Rolled sections		$h/b > 1.2$	$t_f \leq 40 \text{ mm}$	y-y z-z	a b	a_0 a_0
			$40 \text{ mm} < t_f \leq 100$	y-y z-z	b c	a a
		$h/b \leq 1.2$	$t_f \leq 100 \text{ mm}$	y-y z-z	b c	a a
			$t_f > 100 \text{ mm}$	y-y z-z	d d	c c
Welded I-sections		$t_f \leq 40 \text{ mm}$		y-y z-z	b c	b c
		$t_f > 40 \text{ mm}$		y-y z-z	c d	c d
Hollow sections		hot finished		any	a	a_0
		cold formed		any	c	c
Welded box sections		generally (except as below)		any	b	b
		thick welds: $a > 0.5t_f$ $b/t_f < 30$ $h/t_w < 30$		any	c	c
U-, T- and solid sections				any	c	c
L-sections				any	b	b

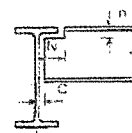
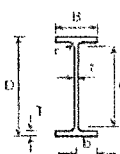
Table S7 Imperfection factors for buckling curves (Table 6.1 of EN 1993-1-1)

Buckling curve	a_0	a	b	c	d
Imperfection factor α	0.13	0.21	0.34	0.49	0.76

1166 Dimensions and properties

Tables of dimensions and gross section properties

UNIVERSAL BEAMS



DIMENSIONS

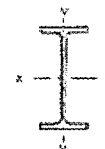
Section Designation	Mass per Metre kg/m	Depth of Section D mm	Width of Section B mm	Thickness		Root Radius r mm	Depth between Fillets d mm	Ratios for Local Buckling		Dimensions for Detailing				Surface Area	
				Web t mm	Flange T mm			Flange b/T	Web d/t	End Clearance C mm	Notch		Per Metre m ²	Per Tonne m ²	
											N mm	n mm			
1016 x 305 x 487 # 1	486.6	1036.1	308.5	30.0	54.1	30.0	867.9	2.85	28.9	17	150	66	3.19	6.57	
1016 x 305 x 437 # 1	436.9	1025.9	305.4	26.9	49.0	30.0	867.9	3.12	32.3	16	150	80	3.17	7.25	
1016 x 305 x 393 # 1	392.7	1016.0	302.0	24.4	43.9	30.0	868.2	3.45	35.6	14	150	74	3.14	8.01	
1016 x 305 x 349 # 1	349.4	1008.1	302.0	21.1	40.0	30.0	868.1	3.77	41.1	13	150	70	3.13	8.96	
1016 x 305 x 314 # 1	314.3	1000.0	300.0	19.1	35.9	30.0	868.2	4.15	45.3	12	150	66	3.11	9.90	
1016 x 305 x 272 # 1	272.3	990.1	300.0	16.5	31.0	30.0	868.1	4.84	52.6	10	152	63	3.10	11.4	
1016 x 305 x 249 # 1	248.7	980.2	300.0	16.5	26.0	30.0	868.2	5.77	52.6	10	152	56	3.08	12.4	
1016 x 305 x 222 # 1	222.0	970.3	300.0	16.0	21.1	30.0	868.1	7.11	54.3	10	152	52	3.06	13.8	
914 x 419 x 385 #	388.0	921.0	420.5	21.4	36.6	24.1	799.6	5.74	37.4	13	210	62	3.44	8.87	
914 x 419 x 343 #	343.3	911.8	418.5	19.4	32.0	24.1	799.6	6.54	41.2	12	210	58	3.42	9.95	
914 x 305 x 289 #	289.1	926.6	307.7	19.5	32.0	19.1	824.4	4.81	42.3	12	156	52	3.01	10.4	
914 x 305 x 253 #	253.4	918.4	305.5	17.3	27.9	19.1	824.4	5.47	47.7	11	156	48	2.99	11.8	
914 x 305 x 224 #	224.2	910.4	304.1	15.9	23.9	19.1	824.4	6.36	51.8	10	156	44	2.97	13.3	
914 x 305 x 201 #	200.9	903.0	303.3	15.1	20.2	19.1	824.4	7.51	54.6	10	156	40	2.96	14.7	
838 x 292 x 226 #	226.5	850.9	293.8	16.1	26.8	17.8	761.7	5.48	47.3	10	150	46	2.81	12.4	
838 x 292 x 194 #	193.8	840.7	292.4	14.7	21.7	17.8	761.7	6.74	51.8	9	150	40	2.79	14.4	
838 x 292 x 176 #	175.9	834.9	291.7	14.0	18.8	17.8	761.7	7.76	54.4	9	150	38	2.78	15.8	
762 x 267 x 197	196.8	769.6	268.0	15.6	25.4	16.5	686.0	5.28	44.0	10	138	42	2.55	13.0	
762 x 267 x 173	173.0	762.2	266.7	14.3	21.6	16.5	686.0	6.17	46.0	9	138	40	2.53	14.6	
762 x 267 x 147	146.9	754.0	265.2	12.8	17.5	16.5	686.0	7.58	53.6	8	138	34	2.51	17.1	
762 x 267 x 134	133.9	750.0	264.4	12.0	15.5	16.5	686.0	8.53	57.2	8	138	32	2.51	18.7	
686 x 254 x 170	170.2	682.9	255.8	14.5	23.7	15.2	615.1	5.40	42.4	9	132	40	2.35	13.8	
686 x 254 x 152	152.4	687.5	254.5	13.2	21.0	15.2	615.1	6.06	46.6	9	132	38	2.34	15.4	
686 x 254 x 140	140.1	683.5	253.7	12.4	19.0	15.2	615.1	6.68	49.6	8	132	36	2.33	16.6	
686 x 254 x 125	125.2	677.9	253.0	11.7	16.2	15.2	615.1	7.81	52.6	8	132	32	2.32	18.5	
610 x 305 x 238	238.1	635.8	311.4	18.4	31.4	16.5	540.0	4.96	29.3	11	158	48	2.45	10.3	
610 x 305 x 179	179.0	620.2	307.1	14.1	23.6	16.5	540.0	6.51	38.3	9	158	42	2.41	13.5	
610 x 305 x 149	149.2	612.4	304.8	11.8	19.7	16.5	540.0	7.74	45.8	8	158	38	2.39	16.0	
610 x 229 x 140	139.9	617.2	230.2	13.1	22.1	12.7	547.6	5.21	41.8	9	120	36	2.11	15.1	
610 x 229 x 125	125.1	612.2	229.0	11.9	19.6	12.7	547.6	5.84	46.0	8	120	34	2.09	16.7	
610 x 229 x 113	113.0	607.6	228.2	11.1	17.3	12.7	547.6	6.60	49.3	8	120	30	2.08	18.4	
610 x 229 x 101	101.2	602.6	227.6	10.5	14.8	12.7	547.6	7.69	52.2	7	120	28	2.07	20.5	
533 x 210 x 122	122.0	544.5	211.9	12.7	21.3	12.7	476.5	4.97	37.5	8	110	34	1.89	15.5	
533 x 210 x 109	109.0	539.5	210.8	11.6	18.8	12.7	476.5	5.61	41.1	8	110	32	1.88	17.2	
533 x 210 x 101	101.0	536.7	210.0	10.8	17.4	12.7	476.5	6.03	44.1	7	110	32	1.87	18.5	
533 x 210 x 92	92.1	533.1	209.3	10.1	15.6	12.7	476.5	6.71	47.2	7	110	30	1.86	20.2	
533 x 210 x 82	82.2	528.3	208.8	9.6	13.2	12.7	476.5	7.91	49.6	7	110	26	1.85	22.5	

† Section is not given in BS 4-1: 1993.
Check availability.

BS 4-1:1993 Universal Beams

Dimensions and properties 1167

UNIVERSAL BEAMS



PROPERTIES

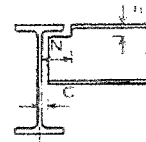
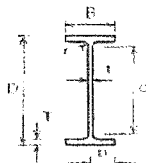
Section Designation	Second Moment of Area		Radius of Gyration		Elastic Modulus		Plastic Modulus		Buckling Parameter	Torsional Index	Warping Constant	Torsional Constant	Area of Section
	Axis X-X cm ⁴	Axis Y-Y cm ⁴	Axis X-X cm	Axis Y-Y cm	Axis X-X cm ³	Axis Y-Y cm ³	Axis X-X cm ³	Axis Y-Y cm ³					
1016 x 305 x 487 #†	1020000	26700	40.6	6.57	19700	1730	23200	2800	0.867	21.1	64.4	4300	620
1016 x 305 x 437 #†	910000	23500	40.4	6.49	17700	1540	20900	2470	0.866	23.1	55.9	3190	557
1016 x 305 x 393 #†	808000	20500	40.2	6.40	15900	1350	18500	2170	0.865	25.5	48.4	2330	500
1016 x 305 x 349 #†	723000	18500	40.3	6.44	14400	1220	16800	1940	0.872	27.9	43.3	1720	445
1016 x 305 x 314 #†	644000	16200	40.1	6.37	12900	1080	14900	1710	0.872	30.7	37.7	1260	400
1016 x 305 x 272 #†	554000	14000	40.0	6.35	11200	934	12800	1470	0.872	35.0	32.2	835	347
1016 x 305 x 249 #†	481000	11800	39.0	6.09	9820	784	11400	1250	0.861	39.9	26.8	582	317
1016 x 305 x 222 #†	408000	9550	38.0	5.81	8410	636	9810	1020	0.849	45.8	21.5	330	283
914 x 419 x 388 #	720000	45400	38.2	9.59	15600	2160	17700	3340	0.885	26.7	68.9	1730	494
914 x 419 x 343 #	626000	39200	37.8	9.46	13700	1870	15500	2890	0.883	30.1	75.8	1190	437
914 x 305 x 289 #	504000	15800	37.6	6.51	10900	1010	12600	1600	0.867	31.9	31.2	928	358
914 x 305 x 253 #	436000	13300	36.8	6.42	9500	871	10900	1370	0.865	36.2	26.4	626	323
914 x 305 x 224 #	376000	11200	36.3	6.27	8270	739	9540	1160	0.861	41.3	22.1	422	286
914 x 305 x 201 #	325000	9420	35.7	6.07	7200	621	8350	982	0.853	46.9	18.4	291	256
838 x 292 x 226 #	340000	11400	34.3	6.27	7890	773	9160	1210	0.859	35.0	19.3	514	269
838 x 292 x 194 #	279000	9070	33.5	6.06	6640	620	7640	974	0.852	41.6	15.2	306	247
838 x 292 x 176 #	246000	7800	33.1	5.90	5890	525	6810	842	0.856	45.5	13.0	221	224
762 x 267 x 187	240000	8180	30.9	5.71	6290	610	7170	959	0.868	33.2	11.3	404	251
762 x 267 x 173	203000	6850	30.5	5.58	5390	514	6200	807	0.865	38.1	9.39	267	220
762 x 267 x 147	169000	5460	30.0	5.40	4470	411	5160	647	0.858	45.2	7.40	159	187
762 x 267 x 134	151000	4780	29.7	5.30	4020	362	4640	570	0.853	49.8	6.46	119	171
686 x 254 x 170	170000	6630	28.0	5.53	4920	518	5630	811	0.872	31.8	7.42	308	217
686 x 254 x 152	150000	5780	27.8	5.46	4370	455	5000	710	0.871	35.4	6.42	220	194
686 x 254 x 140	136000	5180	27.6	5.35	3990	408	4580	626	0.869	38.6	5.72	169	178
686 x 254 x 125	118000	4380	27.2	5.24	3480	346	3990	542	0.863	43.8	4.80	116	159
610 x 305 x 238	210000	15800	26.3	7.23	6590	1020	7490	1570	0.887	21.3	14.5	785	303
610 x 305 x 179	153000	11400	25.8	7.07	4940	743	5550	1140	0.886	27.7	10.2	340	228
610 x 305 x 149	128600	9310	25.7	7.00	4110	611	4580	927	0.886	32.7	8.17	200	190
610 x 229 x 140	112000	4510	25.0	5.09	3620	391	4140	611	0.875	30.6	3.39	216	178
610 x 229 x 125	98600	3930	24.9	4.97	3220	343	3680	535	0.874	34.1	3.45	154	159
610 x 229 x 113	87300	3430	24.6	4.88	2870	301	3280	469	0.870	36.1	2.89	111	144
610 x 229 x 101	75800	2920	24.2	4.75	2520	256	2880	400	0.863	43.1	2.52	77.0	129
533 x 210 x 122	76000	3390	22.1	4.67	2790	320	3200	500	0.878	27.6	2.32	178	155
533 x 210 x 109	66800	2940	21.9	4.60	2460	279	2830	436	0.874	31.0	1.99	126	139
533 x 210 x 101	61900	2690	21.8	4.57	2280	256	2610	399	0.873	33.2	1.81	101	129
533 x 210 x 92	55200	2390	21.7	4.51	2070	228	2360	356	0.873	36.4	1.60	75.7	117
533 x 210 x 82	47500	2010	21.3	4.38	1800	192	2060	300	0.863	41.6	1.33	51.5	105

† Section is not given in BS 4-1:1993.

Check availability.

1168 Dimensions and properties

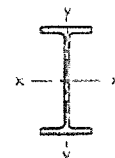
UNIVERSAL BEAMS



DIMENSIONS

Section Designation	Mass per Metre kg/m	Depth of Section D mm	Width of Section B mm	Thickness		Root Radius r mm	Depth between Fillets d mm	Ratios for Local Buckling		Dimensions for Detailing			Surface Area	
				Web t mm	Flange T mm			Flange b/T	Web d/T	End Clearance C mm	Notch		Per Metre m ²	Per Tonne m ²
											N mm	n mm		
457 × 191 × 98	98.3	467.2	192.8	11.4	19.6	10.2	407.6	4.92	35.8	8	102	30	1.67	16.9
457 × 191 × 89	89.3	463.4	191.9	10.5	17.7	10.2	407.6	5.42	38.8	7	102	29	1.66	18.5
457 × 191 × 82	82.0	460.0	191.3	9.9	16.0	10.2	407.6	5.98	41.2	7	102	28	1.65	20.1
457 × 191 × 74	74.3	457.0	190.4	9.0	14.5	10.2	407.6	6.57	45.3	7	102	26	1.64	22.1
457 × 191 × 67	67.1	453.4	189.9	8.5	12.7	10.2	407.6	7.48	48.0	6	102	24	1.63	24.3
457 × 152 × 82	82.1	465.8	155.3	10.5	18.9	10.2	407.6	4.11	38.8	7	84	30	1.51	18.4
457 × 152 × 74	74.2	462.0	154.4	9.6	17.0	10.2	407.6	4.54	42.5	7	84	28	1.50	20.3
457 × 152 × 67	67.2	458.0	153.8	9.0	15.0	10.2	407.6	5.13	45.3	7	84	26	1.50	22.3
457 × 152 × 60	59.8	454.6	152.9	8.1	13.3	10.2	407.6	5.75	50.3	6	84	24	1.49	24.9
457 × 152 × 52	52.3	449.8	152.4	7.6	10.9	10.2	407.6	6.99	53.6	6	84	22	1.48	28.2
406 × 178 × 74	74.2	412.8	179.5	9.5	16.0	10.2	360.4	5.61	37.9	7	96	28	1.51	20.3
406 × 178 × 67	67.1	409.4	178.8	8.8	14.3	10.2	360.4	6.25	41.0	6	96	26	1.50	22.3
406 × 178 × 60	60.1	406.4	177.9	7.9	12.8	10.2	360.4	6.95	45.6	6	96	24	1.49	24.8
406 × 178 × 54	54.1	402.6	177.7	7.7	10.9	10.2	360.4	8.15	46.8	6	96	22	1.48	27.4
406 × 140 × 46	46.0	403.2	140.2	6.8	11.7	10.2	360.4	6.35	53.0	5	78	22	1.34	29.2
406 × 140 × 39	39.0	396.0	141.6	6.4	8.6	10.2	360.4	8.24	56.3	5	78	20	1.33	34.2
356 × 171 × 67	67.1	363.4	173.2	9.1	15.7	10.2	311.6	5.52	34.2	7	94	25	1.38	20.6
356 × 171 × 57	57.0	358.0	172.2	8.1	13.0	10.2	311.6	6.62	38.5	6	94	24	1.37	24.1
356 × 171 × 51	51.0	355.0	171.5	7.4	11.5	10.2	311.6	7.46	42.1	6	94	22	1.36	26.7
356 × 171 × 45	45.0	351.4	171.1	7.0	9.7	10.2	311.6	8.82	44.5	6	94	20	1.36	30.1
356 × 127 × 39	39.1	353.4	126.0	6.6	10.7	10.2	311.6	5.89	47.2	5	70	22	1.18	30.2
356 × 127 × 33	33.1	349.0	125.4	6.0	8.5	10.2	311.6	7.38	51.9	5	70	20	1.17	35.4
305 × 165 × 54	54.0	310.4	166.9	7.9	13.7	8.9	265.2	6.09	33.6	6	90	24	1.26	23.3
305 × 165 × 46	46.1	306.6	165.7	6.7	11.8	8.9	265.2	7.02	39.6	5	90	22	1.25	27.1
305 × 165 × 40	40.3	303.4	165.0	6.0	10.2	8.9	265.2	8.09	44.2	5	90	20	1.24	30.8
305 × 127 × 48	48.1	311.0	125.3	9.0	14.0	8.9	265.2	4.47	29.5	7	70	24	1.09	22.7
305 × 127 × 42	41.9	307.2	124.3	8.0	12.1	8.9	265.2	5.14	39.1	6	70	22	1.08	25.8
305 × 127 × 37	37.0	304.4	123.4	7.1	10.7	8.9	265.2	5.77	37.4	6	70	20	1.07	29.0
305 × 102 × 33	32.8	312.7	102.4	6.6	10.8	7.6	275.9	4.74	41.8	5	58	20	1.01	30.8
305 × 102 × 28	28.2	308.7	101.8	6.0	8.8	7.6	275.9	5.78	46.0	5	58	18	1.00	35.4
305 × 102 × 25	24.8	305.1	101.6	5.8	7.0	7.6	275.9	7.26	47.6	5	58	16	0.992	40.0
254 × 146 × 43	43.0	259.6	147.3	7.2	12.7	7.6	219.0	5.80	30.4	6	82	22	1.08	25.1
254 × 146 × 37	37.0	256.0	146.4	6.3	10.9	7.6	219.0	6.72	34.8	5	82	20	1.07	29.0
254 × 146 × 31	31.1	251.4	146.1	6.0	8.6	7.6	219.0	8.49	36.5	5	82	18	1.06	34.2
254 × 102 × 28	28.3	260.4	102.2	6.3	10.0	7.6	225.2	5.11	35.7	5	58	18	0.904	31.9
254 × 102 × 25	25.2	257.2	101.9	6.0	8.4	7.6	225.2	6.07	37.5	5	58	16	0.897	35.6
254 × 102 × 22	22.0	254.0	101.6	5.7	6.6	7.6	225.2	7.47	39.5	5	58	16	0.890	40.5
203 × 133 × 30	30.0	206.8	133.9	6.4	9.6	7.6	172.4	6.97	26.9	5	74	18	0.923	30.8
203 × 133 × 25	25.1	203.2	133.2	5.7	7.8	7.6	172.4	8.54	30.2	5	74	16	0.915	36.4
203 × 102 × 23	23.1	203.2	101.6	5.4	9.3	7.6	169.4	5.47	31.4	5	60	18	0.790	34.2
178 × 102 × 19	19.0	177.8	101.2	4.8	7.9	7.6	146.8	6.41	30.6	4	60	16	0.738	38.8
152 × 89 × 16	16.0	152.4	88.7	4.5	7.7	7.6	121.8	5.75	27.1	4	54	16	0.638	39.8
127 × 76 × 13	13.0	127.0	76.0	4.0	7.6	7.6	98.6	5.00	24.1	4	46	16	0.537	41.3

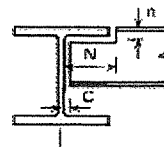
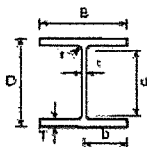
UNIVERSAL BEAMS



PROPERTIES

Section Designation	Second Moment of Area		Radius of Gyration		Elastic Modulus		Plastic Modulus		Buckling Parameter u	Torsional Index x	Warping Constant H dm^6	Torsional Constant J cm^4	Area of Section A cm^2
	Axis $x-x$ cm^4	Axis $y-y$ cm^4	Axis $x-x$ cm	Axis $y-y$ cm	Axis $x-x$ cm^3	Axis $y-y$ cm^3	Axis $x-x$ cm^3	Axis $y-y$ cm^3					
457 x 191 x 98	45700	2350	19.1	4.33	1960	243	2230	379	0.882	25.7	1.18	121	125
457 x 191 x 89	41000	2090	19.0	4.29	1770	218	2010	338	0.879	28.3	1.04	90.7	114
457 x 191 x 82	37100	1870	18.8	4.23	1610	196	1830	304	0.879	30.8	0.922	69.2	104
457 x 191 x 74	33300	1670	18.8	4.20	1460	176	1650	272	0.877	33.8	0.818	51.6	94.6
457 x 191 x 67	29400	1450	18.5	4.12	1300	153	1470	237	0.872	37.9	0.705	37.1	85.5
457 x 152 x 82	36600	1190	18.7	3.37	1570	153	1810	240	0.871	27.4	0.591	59.2	105
457 x 152 x 74	32700	1050	18.6	3.33	1410	136	1630	213	0.873	30.2	0.518	65.9	94.5
457 x 152 x 67	28900	913	18.4	3.27	1260	119	1450	187	0.868	33.6	0.448	47.7	85.6
457 x 152 x 60	25500	795	18.3	3.23	1120	104	1290	163	0.868	37.5	0.387	33.6	76.2
457 x 152 x 52	21400	645	17.9	3.11	950	84.6	1100	133	0.859	43.8	0.311	21.4	66.6
406 x 178 x 74	27300	1550	17.0	4.04	1320	172	1500	267	0.882	27.6	0.608	62.6	94.5
406 x 178 x 67	24300	1370	16.9	3.99	1190	153	1350	237	0.880	30.5	0.533	46.1	85.5
406 x 178 x 60	21600	1200	16.8	3.97	1060	135	1200	209	0.880	33.8	0.466	33.3	76.5
406 x 178 x 54	18700	1020	16.5	3.85	930	115	1060	178	0.871	36.3	0.392	23.1	69.0
406 x 140 x 46	15700	538	16.4	3.03	778	75.7	888	118	0.872	39.0	0.207	19.0	56.6
406 x 140 x 39	12500	410	15.9	2.87	629	57.8	724	90.8	0.858	47.5	0.155	10.7	49.7
356 x 171 x 67	19500	1360	15.1	3.99	1070	157	1210	243	0.886	24.4	0.412	55.7	85.5
356 x 171 x 57	16000	1110	14.9	3.91	896	129	1010	199	0.882	28.6	0.330	33.4	72.6
356 x 171 x 51	14100	968	14.8	3.86	796	113	896	174	0.881	32.1	0.286	23.6	64.9
356 x 171 x 45	12100	811	14.5	3.76	687	94.8	775	147	0.874	36.8	0.237	15.8	57.3
356 x 127 x 39	10200	358	14.3	2.68	576	56.8	659	82.1	0.871	35.2	0.105	15.1	49.8
356 x 127 x 33	8250	280	14.0	2.58	473	44.7	543	70.3	0.863	42.2	0.081	8.79	42.1
305 x 165 x 54	11700	1060	13.0	3.93	754	127	846	198	0.889	23.6	0.234	34.8	68.8
305 x 165 x 46	9900	896	13.0	3.90	646	108	720	166	0.891	27.1	0.195	22.2	58.7
305 x 165 x 40	8500	764	12.9	3.86	560	92.6	623	142	0.889	31.0	0.164	14.7	51.3
305 x 127 x 48	9580	461	12.5	2.74	616	73.6	711	116	0.874	23.3	0.102	31.8	61.2
305 x 127 x 42	8200	389	12.4	2.70	534	62.6	614	98.4	0.872	26.6	0.0846	21.1	53.4
305 x 127 x 37	7170	336	12.3	2.67	471	54.5	539	85.4	0.871	29.7	0.0725	14.8	47.2
305 x 102 x 33	6500	194	12.5	2.15	416	37.9	481	60.0	0.867	31.6	0.0442	12.2	41.8
305 x 102 x 28	5370	155	12.2	2.08	348	30.5	403	46.5	0.858	37.4	0.0349	7.40	35.9
305 x 102 x 25	4460	123	11.9	1.97	292	24.2	342	36.8	0.846	43.4	0.0273	4.77	31.6
254 x 146 x 43	6540	677	10.9	3.52	504	92.0	566	141	0.890	21.2	0.103	23.9	54.8
254 x 146 x 37	5540	571	10.8	3.48	433	78.0	483	119	0.889	24.4	0.0857	15.3	47.2
254 x 146 x 31	4410	446	10.5	3.36	351	61.3	393	94.1	0.879	29.6	0.0660	8.55	39.7
254 x 102 x 28	4010	179	10.5	2.22	308	34.9	353	54.8	0.874	27.5	0.0280	9.57	36.1
254 x 102 x 25	3420	149	10.3	2.15	266	29.2	306	46.0	0.867	31.4	0.0230	6.42	32.0
254 x 102 x 22	2840	119	10.1	2.06	224	23.6	259	37.3	0.856	36.3	0.0182	4.15	28.0
203 x 133 x 30	2900	385	8.71	3.17	280	57.5	318	88.2	0.881	21.6	0.0374	10.3	38.2
203 x 133 x 25	2340	308	8.66	3.10	230	46.2	254	70.9	0.877	25.6	0.0294	5.96	32.0
203 x 102 x 23	2110	164	8.46	2.36	207	32.2	234	49.8	0.868	22.5	0.0154	7.02	29.4
178 x 102 x 19	1360	137	7.48	2.37	153	27.0	171	41.6	0.886	22.6	0.00987	4.41	24.3
152 x 89 x 16	834	89.6	6.41	2.10	109	20.2	123	31.2	0.889	19.6	0.00470	3.56	20.3
127 x 76 x 13	473	55.7	5.35	1.84	74.6	14.7	84.2	22.6	0.896	16.3	0.00199	2.85	16.5

UNIVERSAL COLUMNS



DIMENSIONS

Section Designation	Mass per Metre	Depth of Section	Width of Section	Thickness		Root Radius	Depth between Fillets	Ratios for Local Buckling		Dimensions for Detailing			Surface Area	
				Web	Flange			Flange	Web	End Clearance	Notch		Per Metre	Per Tonne
											C	N		
	kg/m	D mm	B mm	t mm	T mm	r mm	d mm	b/T	d/t	C mm	N mm	n mm	m ²	m ² /t
356 x 406 x 634 #	633.9	474.6	424.0	47.6	77.0	15.2	290.2	2.75	6.10	26	200	94	2.52	3.98
356 x 406 x 551 #	551.0	455.6	418.5	42.1	67.5	15.2	290.2	3.10	6.89	23	200	84	2.47	4.49
356 x 406 x 467 #	467.0	436.6	412.2	35.8	58.0	15.2	290.2	3.55	8.11	20	200	74	2.42	5.19
356 x 406 x 393 #	393.0	419.0	407.0	30.6	49.2	15.2	290.2	4.14	9.48	17	200	66	2.38	6.05
356 x 406 x 340 #	339.9	406.4	403.0	26.6	42.9	15.2	290.2	4.70	10.9	15	200	60	2.35	6.90
356 x 406 x 287 #	287.1	393.6	399.0	22.6	36.5	15.2	290.2	5.47	12.8	13	200	52	2.31	8.05
356 x 406 x 235 #	235.1	381.0	394.8	18.4	30.2	15.2	290.2	6.54	15.8	11	200	46	2.28	9.69
356 x 368 x 202 #	201.9	374.6	374.7	16.5	27.0	15.2	290.2	6.94	17.6	10	190	44	2.19	10.8
356 x 368 x 177 #	177.0	368.2	372.6	14.4	23.8	15.2	290.2	7.89	20.2	9	190	40	2.17	12.3
356 x 368 x 153 #	152.9	362.0	370.5	12.3	20.7	15.2	290.2	8.95	23.6	8	190	36	2.16	14.1
356 x 368 x 129 #	129.0	355.6	368.6	10.4	17.5	15.2	290.2	10.50	27.9	7	190	34	2.14	16.6
305 x 305 x 283	282.9	365.9	322.2	26.8	44.1	15.2	246.7	3.65	9.21	15	158	60	1.94	6.86
305 x 305 x 240	240.0	352.5	318.4	23.0	37.7	15.2	246.7	4.22	10.7	14	158	54	1.91	7.94
305 x 305 x 198	198.1	339.9	314.5	19.1	31.4	15.2	246.7	5.01	12.9	12	158	48	1.87	9.46
305 x 305 x 158	158.1	327.1	311.2	15.8	25.0	15.2	246.7	6.22	15.6	10	158	42	1.84	11.6
305 x 305 x 137	136.9	320.5	309.2	13.8	21.7	15.2	246.7	7.12	17.9	9	158	38	1.82	13.3
305 x 305 x 118	117.9	314.5	307.4	12.0	18.7	15.2	246.7	8.22	20.6	8	158	34	1.81	15.3
305 x 305 x 97	96.9	307.9	305.3	9.9	15.4	15.2	246.7	9.91	24.9	7	158	32	1.79	18.5
254 x 254 x 167	167.1	289.1	265.2	19.2	31.7	12.7	200.3	4.18	10.4	12	134	46	1.58	9.45
254 x 254 x 132	132.0	276.3	261.3	15.3	25.3	12.7	200.3	5.16	13.1	10	134	38	1.55	11.7
254 x 254 x 107	107.1	266.7	258.8	12.8	20.5	12.7	200.3	6.31	15.6	8	134	34	1.52	14.2
254 x 254 x 89	88.9	260.9	256.3	10.3	17.3	12.7	200.3	7.41	19.4	7	134	30	1.50	16.9
254 x 254 x 73	73.1	254.1	254.6	8.6	14.2	12.7	200.3	8.96	23.3	6	134	28	1.49	20.4
203 x 203 x 86	86.1	222.2	209.1	12.7	20.5	10.2	160.8	5.10	12.7	8	110	32	1.24	14.4
203 x 203 x 71	71.0	215.8	206.4	10.0	17.3	10.2	160.8	5.97	16.1	7	110	28	1.22	17.2
203 x 203 x 60	60.0	209.6	205.8	9.4	14.2	10.2	160.8	7.25	17.1	7	110	26	1.21	20.1
203 x 203 x 52	52.0	206.2	204.3	7.9	12.5	10.2	160.8	8.17	20.4	6	110	24	1.20	23.0
203 x 203 x 46	46.1	203.2	203.6	7.2	11.0	10.2	160.8	9.25	22.3	6	110	22	1.19	25.8
152 x 152 x 37	37.0	161.8	154.4	8.0	11.5	7.6	123.6	6.71	15.5	6	84	20	0.912	24.7
152 x 152 x 30	30.0	157.6	152.9	6.5	9.4	7.6	123.6	8.13	19.0	5	84	18	0.901	30.0

PROPERTIES

Section Designation	Second Moment of Area		Radius of Gyration		Elastic Modulus		Plastic Modulus		Buckling Parameter	Torsional Index	Warping Constant	Torsional Constant	Area of Section
	Axis x-x cm ⁴	Axis y-y cm ⁴	Axis x-x cm	Axis y-y cm	Axis x-x cm ²	Axis y-y cm ²	Axis x-x cm ³	Axis y-y cm ³					
356 × 406 × 634 #	275000	98100	18.4	11.0	11600	4630	14200	7110	0.843	5.46	38.8	13700	808
356 × 406 × 551 #	227000	82700	18.0	10.9	9980	3950	12100	6080	0.841	6.05	31.1	9240	702
356 × 406 × 467 #	183000	67800	17.5	10.7	8380	3290	10000	5030	0.839	6.86	24.3	5810	595
356 × 406 × 393 #	147000	55400	17.1	10.5	7000	2720	8220	4150	0.837	7.87	18.9	3550	501
356 × 406 × 340 #	123000	46900	16.8	10.4	6030	2330	7000	3540	0.836	8.84	15.5	2340	433
356 × 406 × 287 #	99900	38700	16.5	10.3	5080	1940	5810	2950	0.834	10.2	12.3	1440	366
356 × 406 × 235 #	79100	31000	16.3	10.2	4160	1570	4680	2380	0.835	12.0	9.54	812	299
356 × 368 × 202 #	66300	23700	16.1	9.60	3540	1260	3970	1920	0.844	13.4	7.16	558	257
356 × 368 × 177 #	57100	20500	15.9	9.54	3100	1100	3480	1670	0.843	15.0	6.09	381	226
356 × 368 × 153 #	48600	17600	15.8	9.49	2680	948	2970	1440	0.844	17.0	5.11	251	195
356 × 368 × 129 #	40300	14600	15.6	9.43	2260	793	2480	1200	0.845	19.8	4.18	153	164
305 × 305 × 283	78900	24600	14.8	8.27	4320	1530	5110	2340	0.856	7.85	6.35	2030	360
305 × 305 × 240	64200	20300	14.5	8.15	3640	1280	4250	1950	0.854	8.74	5.03	1270	306
305 × 305 × 198	50900	16300	14.2	8.04	3000	1040	3440	1580	0.854	10.2	3.68	734	252
305 × 305 × 158	38800	12600	13.9	7.90	2370	808	2680	1230	0.852	12.5	2.87	378	201
305 × 305 × 137	32800	10700	13.7	7.83	2050	692	2300	1050	0.852	14.1	2.39	249	174
305 × 305 × 118	27700	9060	13.6	7.77	1780	589	1960	895	0.851	16.2	1.98	161	150
305 × 305 × 97	22300	7310	13.4	7.69	1460	479	1590	726	0.852	19.2	1.56	91.2	123
254 × 254 × 167	30000	9870	11.9	6.81	2080	744	2420	1140	0.851	8.50	1.63	626	213
254 × 254 × 132	22500	7530	11.6	6.69	1630	576	1870	876	0.850	10.3	1.19	319	168
254 × 254 × 107	17500	5830	11.3	6.59	1310	456	1480	697	0.849	12.4	0.898	172	136
254 × 254 × 89	14300	4860	11.2	6.55	1100	379	1220	575	0.851	14.5	0.717	102	113
254 × 254 × 73	11400	3910	11.1	6.48	898	307	992	465	0.849	17.3	0.562	57.6	93.1
203 × 203 × 86	9450	3130	9.28	5.34	850	299	977	456	0.849	10.2	0.318	137	110
203 × 203 × 71	7620	2540	9.18	5.30	706	246	799	374	0.853	11.9	0.250	80.2	90.4
203 × 203 × 60	6130	2070	8.96	5.20	584	201	656	305	0.846	14.1	0.197	47.2	76.4
203 × 203 × 52	5260	1780	8.91	5.18	510	174	567	264	0.848	15.8	0.167	31.8	66.3
203 × 203 × 46	4570	1550	8.82	5.13	450	152	497	231	0.846	17.7	0.143	22.2	58.7
152 × 152 × 37	2210	706	6.85	3.87	273	91.5	309	140	0.849	13.3	0.0399	19.2	47.1
152 × 152 × 30	1750	560	6.78	3.83	222	73.3	248	112	0.849	16.0	0.0308	10.5	38.3
152 × 152 × 23	1250	400	6.54	3.70	164	52.6	182	80.2	0.840	20.7	0.0212	4.63	29.2